

PRELIMINARY HYDROLOGY & HYDRAULICS STUDY

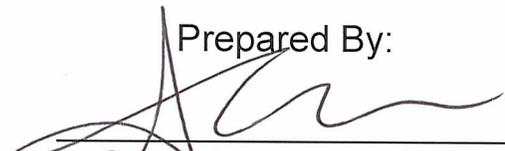
FOR

TENTATIVE TRACT 18186 10460 SLATER AVENUE FOUNTAIN VALLEY, CALIFORNIA

Prepared For:

Coastal Community Fellowship
10460 Slater Avenue
Fountain Valley, California 92708
714.963.9708

Prepared By:


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January 17, 2019

DMS
CONSULTANTS, INC.
CIVIL ENGINEERS

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RATIONAL METHOD HYDROLOGY

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INTRODUCTION AND SUMMARY

Site Description

The proposed Tentative Tract No. 18186 is located at 10460 Slater Avenue and encompasses 2.43 acres in the City of Fountain Valley. The project consists of 12 single family homes varying in size from 4,760 SF to 5,770 SF, and improvements for additional parking spaces for Coastal Community Church. The site is bounded by single family homes to the south and west.

Existing Conditions

The proposed project is located at the southwest corner of Slater Avenue and Ward Street. The overall site is "L" shaped in configuration and relatively flat. The approximate elevations of the site vary from 33.00 to 31.00 feet above mean sea level (msl). Currently the site is occupied by Church buildings and a parking lot.

The project also consists of relocation of existing parking spaces located southerly of the Coastal Community Fellowship (CCF) building to the northerly and westerly side of the CCF building with a new additional access from Slater Avenue. The existing access to CCF facilities from Ward Street will stay in place.

Proposed Conditions

AREA A & B: Residential Portion – Lots 2 to 13 and Lot A (Private Drive)

To capture, store and infiltrate storm water (low flow) runoff will be directed to Eco-Stone permeable pavers underlaid with open graded gravel in Private Drive (Lot A). The flow from permeable pavers will be directed to a bottomless trench drain.

Overflow (high flow) from the area will be directed to Ward Street via a parkway drain.

AREA C: Coastal Community Fellowship Portion – Portion of Lot 1

To capture, store and infiltrate storm water (low flow) runoff will be directed to Eco-Stone permeable pavers underlaid with open graded gravel in drive areas. The flow from permeable pavers will be directed to a bottomless trench drain.

Overflow (high flow) from the area will be directed to Talbert Avenue via a parkway drain.

Purpose

The purpose of this study is to determine the runoff generated by a storm of 25-year and 100-year frequency for both existing and proposed conditions. The site drainage will be designed for a 25-year storm.

Methodology

The hydrology calculations have been prepared using the A.E.S. Program based on the 1986 Orange County Hydrology Manual.

Soil Type

The soil type is Type "B" as determined from Plate A of the Hydrology Manual.

Land Use

For existing conditions, the entire watershed was taken as a church/school. For proposed conditions Area A and Area B were taken as a single family development and Area C was taken as a church/single family development.

Conclusion

The table below summarizes the total runoff from the site for a storm of 25-year and 100-year frequency.

	Existing Conditions	Proposed Conditions
25-Year Storm Event	9.47 cfs	11.22 cfs
100-Year Storm Event	12.31 cfs	14.51 cfs

RATIONAL METHOD HYDROLOGY

Existing Conditions

25-Year Storm Event

RATIONAL METHOD HYDROLOGY COMPUTER PROGRAM PACKAGE
(Reference: 1986 ORANGE COUNTY HYDROLOGY CRITERION)
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Analysis prepared by:

DMS Consultants, Inc.

FILE NAME: 18186EX.DAT
TIME/DATE OF STUDY: 10:16 01/17/2019
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USER SPECIFIED HYDROLOGY AND HYDRAULIC MODEL INFORMATION:
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--*TIME-OF-CONCENTRATION MODEL*--

USER SPECIFIED STORM EVENT (YEAR) = 25.00
SPECIFIED MINIMUM PIPE SIZE (INCH) = 4.00
SPECIFIED PERCENT OF GRADIENTS (DECIMAL) TO USE FOR FRICTION SLOPE = 0.95
DATA BANK RAINFALL USED
ANTECEDENT MOISTURE CONDITION (AMC) I ASSUMED FOR RATIONAL METHOD

USER-DEFINED STREET-SECTIONS FOR COUPLED PIPEFLOW AND STREETFLOW MODEL

NO.	HALF-WIDTH (FT)	CROWN TO CROSSFALL (FT)	STREET-CROSSFALL: IN- / OUT- / SIDE / SIDE / WAY	STREET-CROSSFALL: CURB HEIGHT (FT)	GUTTER-GEOMETRIES: WIDTH (FT)	LIP (FT)	HIKE (FT)	MANNING FACTOR (n)
1	30.0	20.0	0.018/0.018/0.020	0.67	2.00	0.0313	0.167	0.0150

GLOBAL STREET FLOW-DEPTH CONSTRAINTS:

1. Relative Flow-Depth = 0.00 FEET
as (Maximum Allowable Street Flow Depth) - (Top-of-Curb)
2. (Depth)*(Velocity) Constraint = 6.0 (FT*FT/S)

SIZE PIPE WITH A FLOW CAPACITY GREATER THAN OR EQUAL TO THE UPSTREAM TRIBUTARY PIPE.

*USER-SPECIFIED MINIMUM TOPOGRAPHIC SLOPE ADJUSTMENT NOT SELECTED

FLOW PROCESS FROM NODE 101.00 TO NODE 102.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<
=====

INITIAL SUBAREA FLOW-LENGTH (FEET) = 380.00
ELEVATION DATA: UPSTREAM (FEET) = 32.75 DOWNSTREAM (FEET) = 30.43

$T_c = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20$
SUBAREA ANALYSIS USED MINIMUM T_c (MIN.) = 12.293
* 25 YEAR RAINFALL INTENSITY (INCH/HR) = 2.899

SUBAREA T_c AND LOSS RATE DATA (AMC I):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	F_p (INCH/HR)	A_p (DECIMAL)	SCS CN	T_c (MIN.)
SCHOOL	A	1.93	0.40	0.600	17	12.29

SUBAREA AVERAGE PERVIOUS LOSS RATE, F_p (INCH/HR) = 0.40
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, A_p = 0.600
 SUBAREA RUNOFF (CFS) = 4.62
 TOTAL AREA (ACRES) = 1.93 PEAK FLOW RATE (CFS) = 4.62

 FLOW PROCESS FROM NODE 103.00 TO NODE 104.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
 >>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<
 =====

INITIAL SUBAREA FLOW-LENGTH (FEET) = 400.00
 ELEVATION DATA: UPSTREAM (FEET) = 32.70 DOWNSTREAM (FEET) = 31.00

$T_c = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20$

SUBAREA ANALYSIS USED MINIMUM T_c (MIN.) = 13.491

* 25 YEAR RAINFALL INTENSITY (INCH/HR) = 2.759

SUBAREA T_c AND LOSS RATE DATA (AMC I):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	F_p (INCH/HR)	A_p (DECIMAL)	SCS CN	T_c (MIN.)
SCHOOL	A	2.14	0.40	0.600	17	13.49

SUBAREA AVERAGE PERVIOUS LOSS RATE, F_p (INCH/HR) = 0.40

SUBAREA AVERAGE PERVIOUS AREA FRACTION, A_p = 0.600

SUBAREA RUNOFF (CFS) = 4.85

TOTAL AREA (ACRES) = 2.14 PEAK FLOW RATE (CFS) = 4.85
 =====

END OF STUDY SUMMARY:

TOTAL AREA (ACRES) = 2.14 T_c (MIN.) = 13.49

EFFECTIVE AREA (ACRES) = 2.14 AREA-AVERAGED F_m (INCH/HR) = 0.24

AREA-AVERAGED F_p (INCH/HR) = 0.40 AREA-AVERAGED A_p = 0.600

PEAK FLOW RATE (CFS) = 4.85
 =====

END OF RATIONAL METHOD ANALYSIS

RATIONAL METHOD HYDROLOGY

Existing Conditions

100-Year Storm Event

RATIONAL METHOD HYDROLOGY

Proposed Conditions 25-Year Storm Event

RATIONAL METHOD HYDROLOGY COMPUTER PROGRAM PACKAGE
(Reference: 1986 ORANGE COUNTY HYDROLOGY CRITERION)
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USER SPECIFIED HYDROLOGY AND HYDRAULIC MODEL INFORMATION:
=====

--*TIME-OF-CONCENTRATION MODEL*--

USER SPECIFIED STORM EVENT(YEAR) = 100.00
SPECIFIED MINIMUM PIPE SIZE(INCH) = 4.00
SPECIFIED PERCENT OF GRADIENTS(DECIMAL) TO USE FOR FRICTION SLOPE = 0.95
DATA BANK RAINFALL USED
ANTECEDENT MOISTURE CONDITION (AMC) I ASSUMED FOR RATIONAL METHOD

USER-DEFINED STREET-SECTIONS FOR COUPLED PIPEFLOW AND STREETFLOW MODEL

NO.	HALF-WIDTH (FT)	CROWN TO CROSSFALL (FT)	STREET-CROSSFALL IN- / SIDE / SIDE / WAY	STREET-CROSSFALL OUT- / PARK- / WAY	CURB HEIGHT (FT)	GUTTER WIDTH (FT)	GEOMETRIES LIP (FT)	MANNING HIKE (FT)	FACTOR (n)
1	30.0	20.0	0.018/0.018/0.020		0.67	2.00	0.0313	0.167	0.0150

GLOBAL STREET FLOW-DEPTH CONSTRAINTS:

1. Relative Flow-Depth = 0.00 FEET
as (Maximum Allowable Street Flow Depth) - (Top-of-Curb)
2. (Depth)*(Velocity) Constraint = 6.0 (FT*FT/S)

SIZE PIPE WITH A FLOW CAPACITY GREATER THAN OR EQUAL TO THE UPSTREAM TRIBUTARY PIPE.
*USER-SPECIFIED MINIMUM TOPOGRAPHIC SLOPE ADJUSTMENT NOT SELECTED

FLOW PROCESS FROM NODE 101.00 TO NODE 102.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<
=====

INITIAL SUBAREA FLOW-LENGTH(FEET) = 380.00
ELEVATION DATA: UPSTREAM(FEET) = 32.75 DOWNSTREAM(FEET) = 30.43

$T_c = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20$
SUBAREA ANALYSIS USED MINIMUM T_c (MIN.) = 12.293
* 100 YEAR RAINFALL INTENSITY(INCH/HR) = 3.695
SUBAREA T_c AND LOSS RATE DATA(AMC I):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN	Tc (MIN.)
SCHOOL	A	1.93	0.40	0.600	17	12.29

SUBAREA AVERAGE PERVIOUS LOSS RATE, F_p (INCH/HR) = 0.40
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, A_p = 0.600
 SUBAREA RUNOFF(CFS) = 6.00
 TOTAL AREA(ACRES) = 1.93 PEAK FLOW RATE(CFS) = 6.00

 FLOW PROCESS FROM NODE 103.00 TO NODE 104.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
 >>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

=====

INITIAL SUBAREA FLOW-LENGTH(FEET) = 400.00
 ELEVATION DATA: UPSTREAM(FEET) = 32.70 DOWNSTREAM(FEET) = 31.00

$T_c = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20$
 SUBAREA ANALYSIS USED MINIMUM T_c (MIN.) = 13.491
 * 100 YEAR RAINFALL INTENSITY(INCH/HR) = 3.517

SUBAREA T_c AND LOSS RATE DATA(AMC I):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	F_p (INCH/HR)	A_p (DECIMAL)	SCS CN	T_c (MIN.)
SCHOOL	A	2.14	0.40	0.600	17	13.49

SUBAREA AVERAGE PERVIOUS LOSS RATE, F_p (INCH/HR) = 0.40
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, A_p = 0.600
 SUBAREA RUNOFF(CFS) = 6.31
 TOTAL AREA(ACRES) = 2.14 PEAK FLOW RATE(CFS) = 6.31

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END OF STUDY SUMMARY:

TOTAL AREA(ACRES) = 2.14 TC(MIN.) = 13.49
 EFFECTIVE AREA(ACRES) = 2.14 AREA-AVERAGED F_m (INCH/HR) = 0.24
 AREA-AVERAGED F_p (INCH/HR) = 0.40 AREA-AVERAGED A_p = 0.600
 PEAK FLOW RATE(CFS) = 6.31

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END OF RATIONAL METHOD ANALYSIS

RATIONAL METHOD HYDROLOGY

Proposed Conditions

100-Year Storm Event

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DMS Consultants, Inc.

FILE NAME: 18186PR.DAT
TIME/DATE OF STUDY: 10:36 01/17/2019
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USER SPECIFIED HYDROLOGY AND HYDRAULIC MODEL INFORMATION:
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--*TIME-OF-CONCENTRATION MODEL*--

USER SPECIFIED STORM EVENT(YEAR) = 25.00
SPECIFIED MINIMUM PIPE SIZE(INCH) = 4.00
SPECIFIED PERCENT OF GRADIENTS(DECIMAL) TO USE FOR FRICTION SLOPE = 0.95
DATA BANK RAINFALL USED
ANTECEDENT MOISTURE CONDITION (AMC) I ASSUMED FOR RATIONAL METHOD

USER-DEFINED STREET-SECTIONS FOR COUPLED PIPEFLOW AND STREETFLOW MODEL

NO.	HALF- WIDTH (FT)	CROWN TO CROSSFALL (FT)	STREET-CROSSFALL: IN- / OUT- / PARK- SIDE / SIDE / WAY	CURB HEIGHT (FT)	GUTTER-GEOMETRIES: WIDTH LIP HIKE (FT) (FT) (FT)	MANNING FACTOR (n)
1	30.0	20.0	0.018/0.018/0.020	0.67	2.00 0.0313 0.167	0.0150

GLOBAL STREET FLOW-DEPTH CONSTRAINTS:

1. Relative Flow-Depth = 0.00 FEET
as (Maximum Allowable Street Flow Depth) - (Top-of-Curb)
2. (Depth)*(Velocity) Constraint = 6.0 (FT*FT/S)

*SIZE PIPE WITH A FLOW CAPACITY GREATER THAN
OR EQUAL TO THE UPSTREAM TRIBUTARY PIPE.*

*USER-SPECIFIED MINIMUM TOPOGRAPHIC SLOPE ADJUSTMENT NOT SELECTED

FLOW PROCESS FROM NODE 101.00 TO NODE 102.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<
=====

INITIAL SUBAREA FLOW-LENGTH(FEET) = 300.00
ELEVATION DATA: UPSTREAM(FEET) = 33.50 DOWNSTREAM(FEET) = 30.50

$T_c = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20$

SUBAREA ANALYSIS USED MINIMUM Tc(MIN.) = 9.199

* 25 YEAR RAINFALL INTENSITY(INCH/HR) = 3.416

SUBAREA Tc AND LOSS RATE DATA(AMC I):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN	Tc (MIN.)
RESIDENTIAL						

"8-10 DWELLINGS/ACRE" A 1.64 0.40 0.400 17 9.20
 SUBAREA AVERAGE PERVIOUS LOSS RATE, F_p (INCH/HR) = 0.40
 SUBAREA AVERAGE PERVIOUS AREA FRACTION, A_p = 0.400
 SUBAREA RUNOFF (CFS) = 4.81
 TOTAL AREA (ACRES) = 1.64 PEAK FLOW RATE (CFS) = 4.81

 FLOW PROCESS FROM NODE 103.00 TO NODE 104.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
 >>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<
 =====

INITIAL SUBAREA FLOW-LENGTH (FEET) = 300.00
 ELEVATION DATA: UPSTREAM (FEET) = 33.50 DOWNSTREAM (FEET) = 30.50

$T_c = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20$

SUBAREA ANALYSIS USED MINIMUM T_c (MIN.) = 9.199

* 25 YEAR RAINFALL INTENSITY (INCH/HR) = 3.443

SUBAREA T_c AND LOSS RATE DATA (AMC I):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	F_p (INCH/HR)	A_p (DECIMAL)	SCS CN	T_c (MIN.)
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RESIDENTIAL

"8-10 DWELLINGS/ACRE"	A	0.30	0.40	0.400	17	9.20
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SUBAREA AVERAGE PERVIOUS LOSS RATE, F_p (INCH/HR) = 0.40

SUBAREA AVERAGE PERVIOUS AREA FRACTION, A_p = 0.400

SUBAREA RUNOFF (CFS) = 0.89

TOTAL AREA (ACRES) = 0.30 PEAK FLOW RATE (CFS) = 0.89

 FLOW PROCESS FROM NODE 105.00 TO NODE 106.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
 >>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<
 =====

INITIAL SUBAREA FLOW-LENGTH (FEET) = 300.00
 ELEVATION DATA: UPSTREAM (FEET) = 33.20 DOWNSTREAM (FEET) = 30.70

$T_c = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20$

SUBAREA ANALYSIS USED MINIMUM T_c (MIN.) = 10.509

* 25 YEAR RAINFALL INTENSITY (INCH/HR) = 3.181

SUBAREA T_c AND LOSS RATE DATA (AMC I):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	F_p (INCH/HR)	A_p (DECIMAL)	SCS CN	T_c (MIN.)
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SCHOOL	A	1.14	0.40	0.600	17	10.51
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SUBAREA AVERAGE PERVIOUS LOSS RATE, F_p (INCH/HR) = 0.40

SUBAREA AVERAGE PERVIOUS AREA FRACTION, A_p = 0.600

SUBAREA RUNOFF (CFS) = 3.02

TOTAL AREA (ACRES) = 1.14 PEAK FLOW RATE (CFS) = 3.02

 FLOW PROCESS FROM NODE 107.00 TO NODE 108.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
 >>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<
 =====

INITIAL SUBAREA FLOW-LENGTH (FEET) = 300.00
 ELEVATION DATA: UPSTREAM (FEET) = 32.70 DOWNSTREAM (FEET) = 31.00

$T_c = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20$

SUBAREA ANALYSIS USED MINIMUM Tc (MIN.) = 11.352

* 25 YEAR RAINFALL INTENSITY (INCH/HR) = 3.049

SUBAREA Tc AND LOSS RATE DATA (AMC I):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN	Tc (MIN.)
SCHOOL	A	0.99	0.40	0.600	17	11.35

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp (INCH/HR) = 0.40

SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.600

SUBAREA RUNOFF (CFS) = 2.50

TOTAL AREA (ACRES) = 0.99 PEAK FLOW RATE (CFS) = 2.50

=====
END OF STUDY SUMMARY:

TOTAL AREA (ACRES) = 0.99 TC (MIN.) = 11.35

EFFECTIVE AREA (ACRES) = 0.99 AREA-AVERAGED Fm (INCH/HR) = 0.24

AREA-AVERAGED Fp (INCH/HR) = 0.40 AREA-AVERAGED Ap = 0.600

PEAK FLOW RATE (CFS) = 2.50
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END OF RATIONAL METHOD ANALYSIS

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USER SPECIFIED HYDROLOGY AND HYDRAULIC MODEL INFORMATION:
=====

--*TIME-OF-CONCENTRATION MODEL*--

USER SPECIFIED STORM EVENT(YEAR) = 100.00
SPECIFIED MINIMUM PIPE SIZE(INCH) = 4.00
SPECIFIED PERCENT OF GRADIENTS(DECIMAL) TO USE FOR FRICTION SLOPE = 0.95
DATA BANK RAINFALL USED
ANTECEDENT MOISTURE CONDITION (AMC) I ASSUMED FOR RATIONAL METHOD

USER-DEFINED STREET-SECTIONS FOR COUPLED PIPEFLOW AND STREETFLOW MODEL

NO.	HALF- WIDTH (FT)	CROWN TO CROSSFALL (FT)	STREET-CROSSFALL: IN- / OUT- / PARK- SIDE / SIDE / WAY	CURB HEIGHT (FT)	GUTTER-GEOMETRIES: WIDTH (FT)	LIP (FT)	HIKE (FT)	MANNING FACTOR (n)
1	30.0	20.0	0.018/0.018/0.020	0.67	2.00	0.0313	0.167	0.0150

GLOBAL STREET FLOW-DEPTH CONSTRAINTS:

1. Relative Flow-Depth = 0.00 FEET
as (Maximum Allowable Street Flow Depth) - (Top-of-Curb)
2. (Depth)*(Velocity) Constraint = 6.0 (FT*FT/S)

*SIZE PIPE WITH A FLOW CAPACITY GREATER THAN
OR EQUAL TO THE UPSTREAM TRIBUTARY PIPE.*

*USER-SPECIFIED MINIMUM TOPOGRAPHIC SLOPE ADJUSTMENT NOT SELECTED

FLOW PROCESS FROM NODE 101.00 TO NODE 102.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<
=====

INITIAL SUBAREA FLOW-LENGTH(FEET) = 300.00
ELEVATION DATA: UPSTREAM(FEET) = 33.50 DOWNSTREAM(FEET) = 30.50

Tc = K*[(LENGTH** 3.00)/(ELEVATION CHANGE)]**0.20
SUBAREA ANALYSIS USED MINIMUM Tc(MIN.) = 9.199
* 100 YEAR RAINFALL INTENSITY(INCH/HR) = 4.363

SUBAREA Tc AND LOSS RATE DATA(AMC I):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN	Tc (MIN.)
RESIDENTIAL						

"8-10 DWELLINGS/ACRE" A 1.64 0.40 0.400 17 9.20
SUBAREA AVERAGE PERVIOUS LOSS RATE, F_p (INCH/HR) = 0.40
SUBAREA AVERAGE PERVIOUS AREA FRACTION, A_p = 0.400
SUBAREA RUNOFF (CFS) = 6.20
TOTAL AREA (ACRES) = 1.64 PEAK FLOW RATE (CFS) = 6.20

FLOW PROCESS FROM NODE 103.00 TO NODE 104.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

=====

INITIAL SUBAREA FLOW-LENGTH (FEET) = 300.00
ELEVATION DATA: UPSTREAM (FEET) = 33.50 DOWNSTREAM (FEET) = 30.50

$T_c = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20$
SUBAREA ANALYSIS USED MINIMUM T_c (MIN.) = 9.199
* 100 YEAR RAINFALL INTENSITY (INCH/HR) = 4.397

SUBAREA T_c AND LOSS RATE DATA (AMC I):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	F_p (INCH/HR)	A_p (DECIMAL)	SCS CN	T_c (MIN.)
RESIDENTIAL						
"8-10 DWELLINGS/ACRE"	A	0.30	0.40	0.400	17	9.20

SUBAREA AVERAGE PERVIOUS LOSS RATE, F_p (INCH/HR) = 0.40
SUBAREA AVERAGE PERVIOUS AREA FRACTION, A_p = 0.400
SUBAREA RUNOFF (CFS) = 1.14
TOTAL AREA (ACRES) = 0.30 PEAK FLOW RATE (CFS) = 1.14

FLOW PROCESS FROM NODE 105.00 TO NODE 106.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

=====

INITIAL SUBAREA FLOW-LENGTH (FEET) = 300.00
ELEVATION DATA: UPSTREAM (FEET) = 33.20 DOWNSTREAM (FEET) = 30.70

$T_c = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20$
SUBAREA ANALYSIS USED MINIMUM T_c (MIN.) = 10.509
* 100 YEAR RAINFALL INTENSITY (INCH/HR) = 4.058

SUBAREA T_c AND LOSS RATE DATA (AMC I):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	F_p (INCH/HR)	A_p (DECIMAL)	SCS CN	T_c (MIN.)
SCHOOL	A	1.14	0.40	0.600	17	10.51

SUBAREA AVERAGE PERVIOUS LOSS RATE, F_p (INCH/HR) = 0.40
SUBAREA AVERAGE PERVIOUS AREA FRACTION, A_p = 0.600
SUBAREA RUNOFF (CFS) = 3.92
TOTAL AREA (ACRES) = 1.14 PEAK FLOW RATE (CFS) = 3.92

FLOW PROCESS FROM NODE 107.00 TO NODE 108.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<
>>USE TIME-OF-CONCENTRATION NOMOGRAPH FOR INITIAL SUBAREA<<

=====

INITIAL SUBAREA FLOW-LENGTH (FEET) = 300.00
ELEVATION DATA: UPSTREAM (FEET) = 32.70 DOWNSTREAM (FEET) = 31.00

$T_c = K * [(LENGTH ** 3.00) / (ELEVATION CHANGE)] ** 0.20$

SUBAREA ANALYSIS USED MINIMUM Tc (MIN.) = 11.352

* 100 YEAR RAINFALL INTENSITY (INCH/HR) = 3.890

SUBAREA Tc AND LOSS RATE DATA (AMC I):

DEVELOPMENT TYPE/ LAND USE	SCS SOIL GROUP	AREA (ACRES)	Fp (INCH/HR)	Ap (DECIMAL)	SCS CN	Tc (MIN.)
SCHOOL	A	0.99	0.40	0.600	17	11.35

SUBAREA AVERAGE PERVIOUS LOSS RATE, Fp (INCH/HR) = 0.40

SUBAREA AVERAGE PERVIOUS AREA FRACTION, Ap = 0.600

SUBAREA RUNOFF (CFS) = 3.25

TOTAL AREA (ACRES) = 0.99 PEAK FLOW RATE (CFS) = 3.25

=====
END OF STUDY SUMMARY:

TOTAL AREA (ACRES) = 0.99 TC (MIN.) = 11.35

EFFECTIVE AREA (ACRES) = 0.99 AREA-AVERAGED Fm (INCH/HR) = 0.24

AREA-AVERAGED Fp (INCH/HR) = 0.40 AREA-AVERAGED Ap = 0.600

PEAK FLOW RATE (CFS) = 3.25
=====

=====
END OF RATIONAL METHOD ANALYSIS

HYDRAULICS

Parkway Culverts Capacity Calculations

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Ver. 10.2 Release Date: 01/01/2005 License ID 1570

Analysis prepared by:

DMS Consultants, Inc.

TIME/DATE OF STUDY: 14:45 01/17/2019
=====

Problem Descriptions:
Parkway Culvert Calculation

>>>>CHANNEL INPUT INFORMATION<<<<

NORMAL DEPTH(FEET) = 0.33
CHANNEL Z1(HORIZONTAL/VERTICAL) = 0.00
Z2(HORIZONTAL/VERTICAL) = 0.00
BASEWIDTH(FEET) = 2.00
CONSTANT CHANNEL SLOPE(FEET/FEET) = 0.020000
MANNINGS FRICTION FACTOR = 0.0150
=====

NORMAL-DEPTH FLOW INFORMATION:

>>>> NORMAL DEPTH FLOW(CFS) = 3.65
FLOW TOP-WIDTH(FEET) = 2.00
FLOW AREA(SQUARE FEET) = 0.66
HYDRAULIC DEPTH(FEET) = 0.33
FLOW AVERAGE VELOCITY(FEET/SEC.) = 5.53
UNIFORM FROUDE NUMBER = 1.697
PRESSURE + MOMENTUM(POUNDS) = 45.94
AVERAGED VELOCITY HEAD(FEET) = 0.475
SPECIFIC ENERGY(FEET) = 0.805
=====

CRITICAL-DEPTH FLOW INFORMATION:

CRITICAL FLOW TOP-WIDTH(FEET) = 2.00
CRITICAL FLOW AREA(SQUARE FEET) = 0.94
CRITICAL FLOW HYDRAULIC DEPTH(FEET) = 0.47
CRITICAL FLOW AVERAGE VELOCITY(FEET/SEC.) = 3.90
CRITICAL DEPTH(FEET) = 0.47
CRITICAL FLOW PRESSURE + MOMENTUM(POUNDS) = 41.27
AVERAGED CRITICAL FLOW VELOCITY HEAD(FEET) = 0.236
CRITICAL FLOW SPECIFIC ENERGY(FEET) = 0.704
=====

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Analysis prepared by:

DMS Consultants, Inc.

TIME/DATE OF STUDY: 14:48 01/17/2019
=====

Problem Descriptions:
Parkway Calculation

>>>>CHANNEL INPUT INFORMATION<<<<

NORMAL DEPTH(FEET) = 0.33
CHANNEL Z1(HORIZONTAL/VERTICAL) = 0.00
Z2(HORIZONTAL/VERTICAL) = 0.00
BASEWIDTH(FEET) = 3.00
CONSTANT CHANNEL SLOPE(FEET/FEET) = 0.020000
MANNINGS FRICTION FACTOR = 0.0150
=====

NORMAL-DEPTH FLOW INFORMATION:

>>>>> NORMAL DEPTH FLOW(CFS) = 5.80
FLOW TOP-WIDTH(FEET) = 3.00
FLOW AREA(SQUARE FEET) = 0.99
HYDRAULIC DEPTH(FEET) = 0.33
FLOW AVERAGE VELOCITY(FEET/SEC.) = 5.86
UNIFORM FROUDE NUMBER = 1.798
PRESSURE + MOMENTUM(POUNDS) = 76.07
AVERAGED VELOCITY HEAD(FEET) = 0.533
SPECIFIC ENERGY(FEET) = 0.863
=====

CRITICAL-DEPTH FLOW INFORMATION:

CRITICAL FLOW TOP-WIDTH(FEET) = 3.00
CRITICAL FLOW AREA(SQUARE FEET) = 1.46
CRITICAL FLOW HYDRAULIC DEPTH(FEET) = 0.49
CRITICAL FLOW AVERAGE VELOCITY(FEET/SEC.) = 3.97
CRITICAL DEPTH(FEET) = 0.49
CRITICAL FLOW PRESSURE + MOMENTUM(POUNDS) = 66.84
AVERAGED CRITICAL FLOW VELOCITY HEAD(FEET) = 0.244
CRITICAL FLOW SPECIFIC ENERGY(FEET) = 0.732
=====

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Analysis prepared by:

TIME/DATE OF STUDY: 14:51 01/17/2019
=====

Problem Descriptions:
Parkway Calculation

>>>>CHANNEL INPUT INFORMATION<<<<

NORMAL DEPTH(FEET) = 0.33
CHANNEL Z1(HORIZONTAL/VERTICAL) = 0.00
Z2(HORIZONTAL/VERTICAL) = 0.00
BASEWIDTH(FEET) = 1.00
CONSTANT CHANNEL SLOPE(FEET/FEET) = 0.020000
MANNINGS FRICTION FACTOR = 0.0150
=====

NORMAL-DEPTH FLOW INFORMATION:

>>>>> NORMAL DEPTH FLOW(CFS) = 1.57
FLOW TOP-WIDTH(FEET) = 1.00
FLOW AREA(SQUARE FEET) = 0.33
HYDRAULIC DEPTH(FEET) = 0.33
FLOW AVERAGE VELOCITY(FEET/SEC.) = 4.77
UNIFORM FROUDE NUMBER = 1.464
PRESSURE + MOMENTUM(POUNDS) = 17.96
AVERAGED VELOCITY HEAD(FEET) = 0.354
SPECIFIC ENERGY(FEET) = 0.684
=====

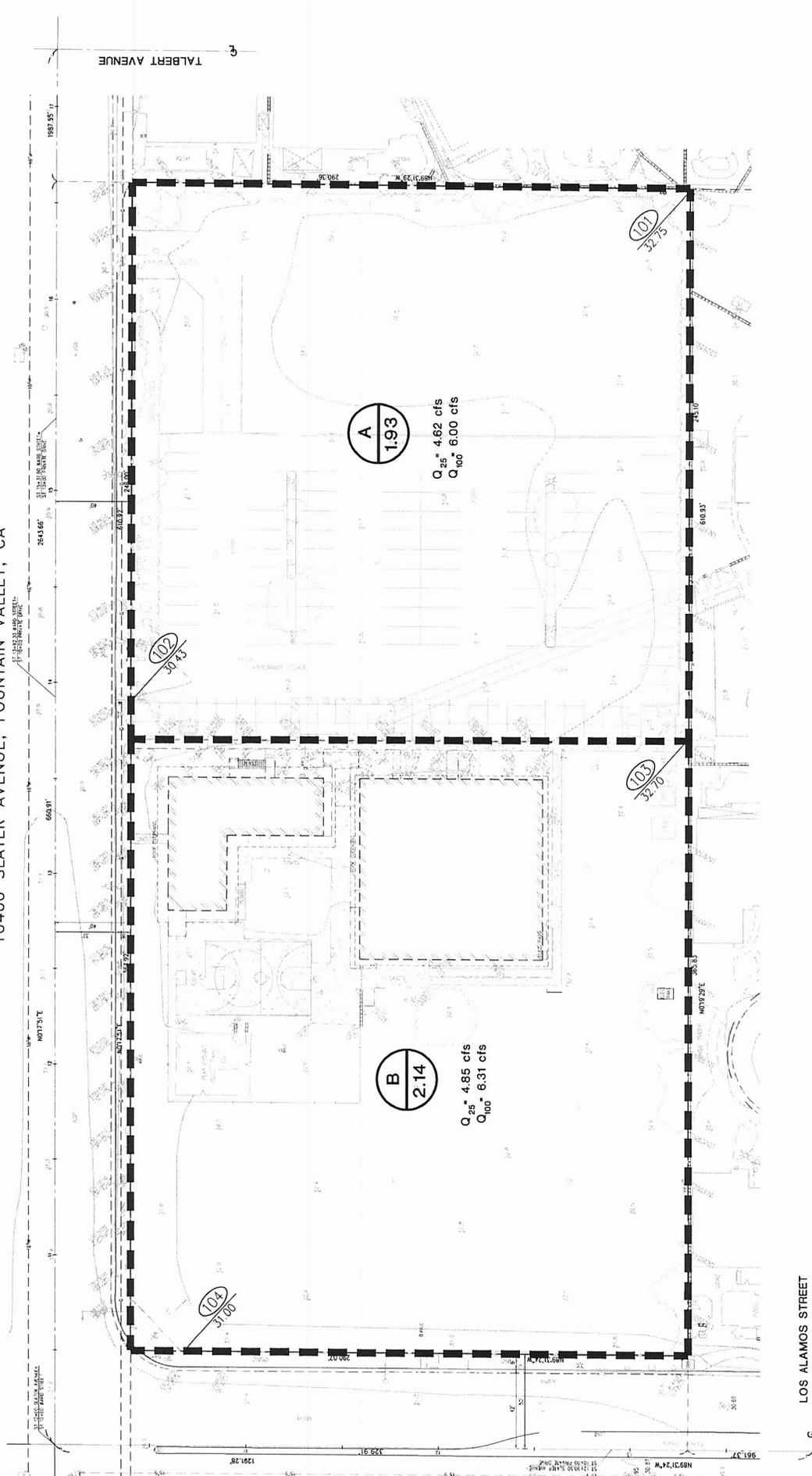
CRITICAL-DEPTH FLOW INFORMATION:

CRITICAL FLOW TOP-WIDTH(FEET) = 1.00
CRITICAL FLOW AREA(SQUARE FEET) = 0.43
CRITICAL FLOW HYDRAULIC DEPTH(FEET) = 0.43
CRITICAL FLOW AVERAGE VELOCITY(FEET/SEC.) = 3.70
CRITICAL DEPTH(FEET) = 0.43
CRITICAL FLOW PRESSURE + MOMENTUM(POUNDS) = 16.94
AVERAGED CRITICAL FLOW VELOCITY HEAD(FEET) = 0.213
CRITICAL FLOW SPECIFIC ENERGY(FEET) = 0.638
=====

APPENDIX
Hydrology Maps
Existing and Proposed Conditions

HYDROLOGY MAP

IN THE CITY OF FOUNTAIN VALLEY, COUNTY OF ORANGE, STATE OF CALIFORNIA
10460 SLATER AVENUE, FOUNTAIN VALLEY, CA



- LEGEND**
- AREA DESIGNATION
 - AREA IN ACRES
 - NODE DESIGNATION
 - 45.00 ELEVATION
 - DIRECTION OF FLOW



SOILS ENGINEER:
REGIONAL CONSULTANT
10460 SLATER AVENUE
FOUNTAIN VALLEY, CA 92708
BY: WHEAT-CHAMBERS

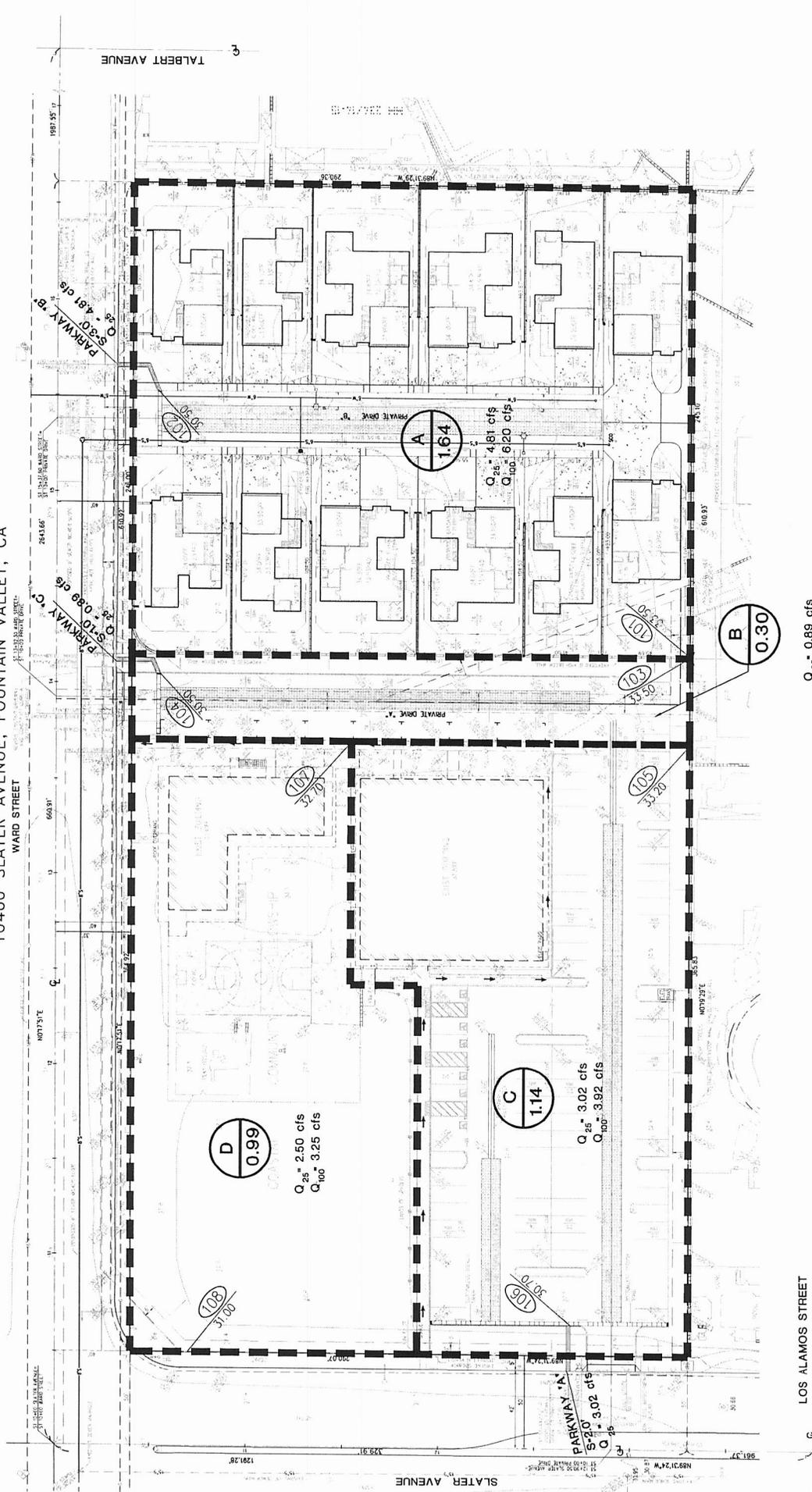
PREPARED FOR:
COASTAL COMMUNITY FELLOWSHIP
10460 SLATER AVENUE
FOUNTAIN VALLEY, CA 92708

PREPARED BY
DMS CONSULTANTS, INC.
10460 SLATER AVENUE
FOUNTAIN VALLEY, CA 92708
TEL: 714.251.1100
WWW.DMSCONSULTANTS.COM

HYDROLOGY MAP - EXISTING CONDITIONS
TENTATIVE TRACT NO. 18188
COASTAL COMMUNITY FELLOWSHIP
10460 SLATER AVENUE
FOUNTAIN VALLEY, CA 92708
DATE: 07/17/09
SHEET 1 OF 1

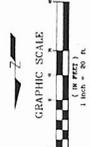
HYDROLOGY MAP

IN THE CITY OF FOUNTAIN VALLEY, COUNTY OF ORANGE, STATE OF CALIFORNIA
 10460 SLATER AVENUE, FOUNTAIN VALLEY, CA



Q₂₅ = 0.89 cfs
 Q₁₀₀ = 1.14 cfs

- LEGEND**
- A AREA DESIGNATION
 - 0.00 AREA IN ACRES
 - XXX NODE DESIGNATION
 - 45.00 ELEVATION
 - DIRECTION OF FLOW



APR. 1981-10-84

PREPARED BY: **DMS CONSULTANTS, INC.**
 10000 WARD STREET, SUITE 100
 FOUNTAIN VALLEY, CA 92708
 TEL: (714) 952-1100

HYDROLOGY MAP - PROPOSED CONDITIONS

TENTATIVE TRACT NO. 18188
 COASTAL COMMUNITY FELLOWSHIP
 FOUNTAIN VALLEY, CA 92708

DATE: 12-11-78

SHEET 1 OF 1

PREPARED FOR:
 COASTAL COMMUNITY FELLOWSHIP
 10000 WARD STREET, SUITE 100
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